

To: Mike Walker, C.E.T.  
Development Review Officer  
Town of Carleton Place, Ontario

From: Alyssa Gladish, E.I.T.  
Project Manager, Community  
Development  
Stantec Ottawa, Ontario

Project/File: 160402061

Date: December 13, 2024

---

**Reference: 124 Moore Street - Carleton Place - Stormwater Management Design Brief Revision 1**

Stantec Consulting Ltd. has been retained by Spurline Dental to prepare this Stormwater Management (SWM) design brief in support of a Development Permit (DP) application for the proposed addition to the commercial development (Dental Office) at 124 Moore Street in the Town of Carleton Place.

**Background**

The 0.16 ha site is situated on the southwest side of Moore Street, near the Rochester Street, Santiago Street, and Miguel Street Intersections with Moore Street (Franktown Road to the southeast). As shown in Figure 1.0. The site is bound to the northeast by Moore Street, and a lane to the south. The surrounding developments are residential to the north and southeast, and commercial to the east and south.

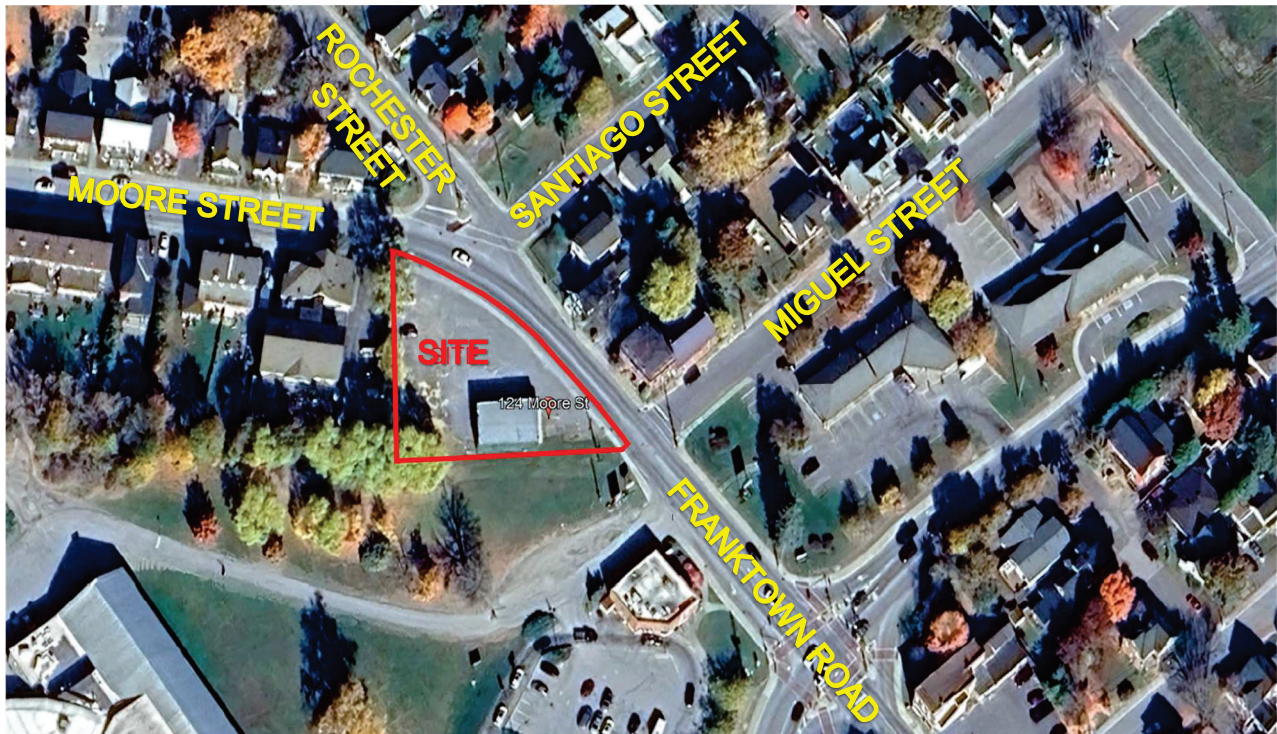


Figure 1 – Site Location

Reference: 124 Moore Street - Carleton Place - Stormwater Management Design Brief Revision 1

The DP application is for a 108.6 m<sup>2</sup>, 1-storey addition on the west side of the existing 1-storey commercial building. The Site Plan includes a re-designed paved parking lot with concrete curbs and soft landscaped areas. Refer to the attached **Site Plan** prepared by Stewart +TSAI Architects Inc. (Revision 03, Issued for Review and Coordination October 30, 2024) for details.

### Stormwater Management

The Town of Carleton Place requires stormwater management quantity control for this development. The 5-year and 100-year post-development release rates from the site are to be limited to the 5-year and 100-year predevelopment release rates; respectively.

### Existing Conditions

The site consists of an existing building with a mix of gravel and asphalt parking areas. The existing drainage patterns are shown in the attached **Existing Drainage Plan (EXSD-1)** drawing. The site drains predominantly from south to north to the Moore Street municipal right of way (ROW), as shown by the existing storm drainage catchment SITE 1. The entire site drains uncontrolled. The pre-development 5-year runoff coefficient (C) is determined to be 0.65 for the 0.16 ha site.

The peak pre-development flow rates shown in **Table 1** have been calculated using the rational method as follows:

$$Q = 2.78 CiA$$

Where:

Q = peak flow rate, L/s

C = runoff coefficient

i = rainfall intensity, mm/hr (as per the City of Ottawa IDF curves)

A = drainage area, ha

*Table 1: Peak Pre-Development Flow Rates*

Design Storm	Pre-Development Flow Rate (L/s) for C=0.65, A=0.16 ha, t <sub>c</sub> =10 min
5-year	30.3
100-year	64.8

Note that the SITE 1 C-value is increased by 25% for the post-development 100-year event as per the MTO Hydrotechnical Design Chart 1.07, January 2023.

Reference: 124 Moore Street - Carleton Place - Stormwater Management Design Brief Revision 1

## Post-Development Conditions

The post-development drainage is shown in the attached **Site Drainage Plan (SD-1)** drawing, where the site is split into three drainage areas, UNC-1, UNC-2, and C100A. Properties of the drainage areas are summarized in the attached **Storm Drainage Area Summary Sheet**.

- UNC-1 drains uncontrolled to the south and ultimately outlets to a grassed swale in the municipal Park Lot 18 and Park Lot 19.
- UNC-2 drains uncontrolled to the north and outlets directly to the Moore Street ROW.
- C100A drains to a grassed swale and into catchbasin C100A-1 where onsite storage is achieved through surface ponding. C100A-1 outlets to the existing 300mm municipal storm sewer in the Moore Street ROW.

Compared to the pre-development conditions, the post-development site has a greater paved parking area (some gravel parking areas to be paved), increased roof areas, and increased soft landscaped areas. The roof area of the existing building drains from front to back (north to south) while the roof area of the building addition is expected to be peaked, draining drain half to the south and half to the north with roof downspouts. The exact locations of the downspouts are to be confirmed in the architectural drawings. Refer to the **Grading Plan (GP-1)** drawing for post-development grades, slopes, and drainage patterns.

### ***Onsite Stormwater Detention***

To restrict the post-development stormwater runoff to the pre-development conditions, outflow from catchbasin C100A-1 is to be restricted by an inlet control device (ICD) to achieve onsite stormwater detention within the grass swale and catch basin. The ICD circular orifice has been sized using a modified rational method (MRM) analysis and the orifice equation for High Flow (HF) ICDs. Refer to the attached **Modified Rational Method Design Sheet** for calculation details.

The properties of the onsite storage area are shown in drawing **SD-1** and are summarized as follows:

- Surface ponding area: 31.5m<sup>2</sup>
- Maximum depth of storage: 0.26m
- Surface storage volume: 4.4m<sup>3</sup>
- 1200mm diameter catchbasin storage volume: 1.4m<sup>3</sup>
- Spillover elevation: 138.14m

The following assumptions were made in the MRM analysis:

- The release rate from catchbasin C100A-1 (*Q<sub>release</sub>*) was determined to be the lesser of the peak release rate from the ICD or the actual release rate of the site (*Q<sub>actual</sub>*).
- Stormwater runoff rates exceeding the peak orifice release rate was assumed to spill uncontrolled from C100A into the Moore Street ROW. This is represented in the MRM sheet as *Q<sub>spill</sub>*.

Reference: 124 Moore Street - Carleton Place - Stormwater Management Design Brief Revision 1

- The downstream water level was set by the properties of the 300 mm diameter receiving municipal storm sewer. Based on our conversation December 5, 2024, this pipe is an upstream run of pipe and is typically dry. Hence, the downstream water level was assumed to be below the proposed invert of the catch basin (136.68).
- Due to the upstream location of the storm sewer, the Town does not anticipate surcharge from the municipal storm sewer into the onsite catchbasin.
- An OPSD 705.010 Type B standard 1200 mm diameter catch basin was assumed for the elevation of the catchbasin rim relative to the invert of the ICD and outgoing pipe.

### ***Inlet Control Device***

To achieve the stormwater detention and release rate targets for this site, a 79 mm diameter circular orifice HF ICD is required. With a maximum hydraulic head of 1.46m, the orifice has a peak outflow rate of 15.0 L/s. The ICD schedule is shown in Table 2.

Table 2: ICD Schedule

CB Name	Diameter (mm)	ICD Type	Invert (m)	5-Year Head (m)	5-Year Peak Flow (L/s)	100-Year Head (m)	100-Year Peak Flow (L/s)
C100A-1	79	High Flow (HF) Circular Orifice	136.68	1.46	15.0	1.46	15.0

### ***Overall Site Stormwater Management***

Considering the controlled and uncontrolled post-development storm drainage areas for this site and the total available 5.6m<sup>3</sup> of onsite storage, the post-development drainage peak release rates and storage volumes are summarized in Table 3. The results demonstrate that the stormwater management plan for the site achieved the design criteria for post-development to pre-development discharge rate control.

Table 3: Peak Post-Development Stormwater Runoff Discharge Rate and Storage Volume Summary

Subcatchment Area	5-Year Discharge (L/s)	5-Year Storage (m <sup>3</sup> )	100-Year Discharge (L/s)	100-Year Storage (m <sup>3</sup> )
C100A	15.0	3.0	15.0	5.8
<b>Subtotal Controlled</b>	<b>15.0</b>	<b>3.0</b>	<b>15.0</b>	<b>5.8</b>
UNC-1	11.0	-	23.5	-
UNC-2	3.1	-	6.6	-
C100A (spillover)	0.0	-	18.3	-
<b>Subtotal Uncontrolled</b>	<b>14.0</b>	<b>-</b>	<b>48.3</b>	<b>-</b>
<b>Total</b>	<b>29.1</b>	<b>3.0</b>	<b>63.3</b>	<b>5.8</b>
<b>Target</b>	<b>30.3</b>	<b>-</b>	<b>64.8</b>	<b>-</b>


Reference: 124 Moore Street - Carleton Place - Stormwater Management Design Brief Revision 1

## Closing

The stormwater management strategy presented in this memo demonstrates that the stormwater design criteria have been met. The proposed development of the 124 Moore Street addition and site improvements will not impact the municipal storm sewer or downstream system by restricting the 5-year and 100-year post-development runoff release rates to the 5-year and 100-year pre-development runoff release rates, respectively. We believe this demonstrates adequate stormwater servicing to support the Development Application Approval. If you have any questions or concerns, please contact the undersigned.

Sincerely,

**Stantec Consulting Ltd.**

 2024.12.13  
12:16:27 -05'00'

---

**Alyssa Gladish** E.I.T.  
Project Manager, Community Development  
Phone: (780) 917-8567  
alyssa.gladish@stantec.com

[stantec.com](https://www.stantec.com)

Attachments:

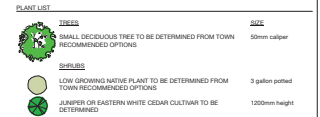
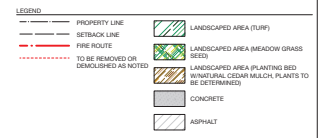
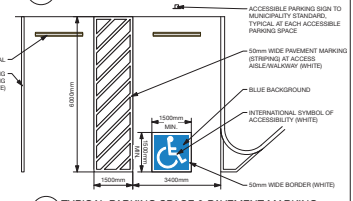
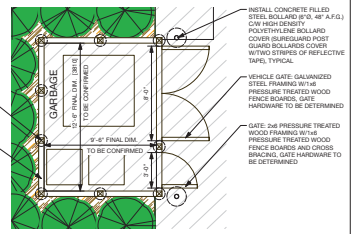
- 01 – Site Plan
- 02 – Storm Drainage Area Summary Sheet
- 03 – Modified Rational Method Design Sheet
- 04 – Storm Sewer Design Sheet
- 05 – Grading Plan (GP-1)
- 06 – Existing Drainage Plan (EXSD-1)
- 07 – Site Drainage Plan (SD-1)



---

**Karin Smadella** P.Eng.  
Principal, Operations Leader, Community Development  
Phone: (613) 724-4371  
karin.smadella@stantec.com





ISSUE / REVISION DATE

01 REVIEW & COORDINATION OCT 13, 2024

02 CARLETON PLACE REVIEW OCT 15, 2024

03 REVIEW & COORDINATION OCT 30, 2024

04 REVIEW & COORDINATION DEC 11, 2024

PROJECT NAME

**DENTAL OFFICE FIT-UP AND BUILDING ADDITION**  
 124 MOORE STREET  
 CARLETON PLACE, ON

SHEET TITLE

**SITE PLAN**

© 2024 ALL RIGHTS RESERVED. Any unauthorized use of these drawings without written permission of Stewart + Tsai Architects Inc. is prohibited. All dimensions are in millimeters unless otherwise noted.

PROJECT NO: 24016

DRAWN BY: OS

CHECKED BY: OS

DATE: 2024 Dec 11

24016-01 (Release 1)

### Stormwater Management Calculations

File No: 160402061  
 Project: 124 Moore Street Addition  
 Date Created: 05-Nov-24  
 Created By: ALG

Rev: 1  
 Checked By: KS  
 Date Checked: 13-Dec-24  
 Revised By: ALG  
 Revision Date: 13-Dec-24

SWM Approach:  
 Post-development to Pre-development flows (5yr to 5yr and 100yr to 100yr)

**Post-Development Site Conditions:**

**Overall Runoff Coefficient for Site and Sub-Catchment Areas**

Runoff Coefficient Table									
Catchment Type	Sub-catchment Area		ID / Description	Area (ha)		Runoff Coefficient "C"		"A x C"	Overall Runoff Coefficient
	Area			"A"		"C"			
Controlled - Tributary		C100A	Hard	0.073		0.9	0.065		
			Soft	0.018		0.2	0.004		
			Subtotal			0.091			
Uncontrolled - Non-Tributary		UNC-2	Hard	0.011		0.9	0.010		
			Soft	0.004		0.2	0.001		
			Subtotal			0.015			
Uncontrolled - Non-Tributary		UNC-1	Hard	0.038		0.9	0.035		
			Soft	0.016		0.2	0.003		
			Subtotal			0.055			
<b>Total</b>					<b>0.161</b>		<b>0.118</b>		<b>0.73</b>
<b>Overall Runoff Coefficient= C:</b>									

Total Roof Areas	0.000 ha
Total Tributary Surface Areas (Controlled and Uncontrolled)	0.091 ha
Total Tributary Area to Outlet	0.091 ha
Total Uncontrolled Areas (Non-Tributary)	0.070 ha
Total Site	0.161 ha

# Stormwater Management Calculations

## Project #160402061, 124 Moore Street Addition Modified Rational Method Calculations for Storage

<b>5 yr Intensity</b> City of Ottawa	$I = a/(t + b)^c$		<b>t (min)</b> 5 10 15 20 25 30 35 40 45 50 55 60	<b>I (mm/hr)</b> 141.18 104.19 83.56 70.25 60.90 53.93 48.52 44.18 40.63 37.65 35.12 32.94
	a =	998.071		
	b =	6.053		
	c =	0.814		

<b>100 yr Intensity</b> City of Ottawa	$I = a/(t + b)^c$		<b>t (min)</b> 5 10 15 20 25 30 35 40 45 50 55 60	<b>I (mm/hr)</b> 242.70 178.56 142.89 119.95 103.85 91.87 82.58 75.15 69.05 63.95 59.62 55.89
	a =	1735.688		
	b =	6.014		
	c =	0.820		

**5 YEAR Predevelopment Target Release from Portion of Site**

Subdrainage Area: Predevelopment Tributary Area to Outlet  
 Area (ha): 0.161  
 C: 0.65

Typical Time of Concentration

tc (min)	I (5 yr) (mm/hr)	Qtarget (L/s)
10	104.19	30.26

**100 YEAR Predevelopment Target Release from Portion of Site**

Subdrainage Area: Predevelopment Tributary Area to Outlet  
 Area (ha): 0.161  
 C: 0.81

Estimated Time of Concentration after Development

tc (min)	I (100 yr) (mm/hr)	Q100yr (L/s)
10	178.56	64.82

**5 YEAR Modified Rational Method for Entire Site**

Subdrainage Area: C100A  
 Area (ha): 0.09  
 C: 0.76

Controlled - Tributary

tc (min)	I (5 yr) (mm/hr)	Qactual (L/s)	Qrelease (L/s)	Qstored (L/s)	Vstored (m³)
10	104.19	20.00	15.01	5.00	3.00
15	83.56	16.04	15.01	1.03	0.93
20	70.25	13.49	13.49	0.00	0.00
25	60.90	11.69	11.69	0.00	0.00
30	53.93	10.35	10.35	0.00	0.00
35	48.52	9.31	9.31	0.00	0.00
40	44.18	8.48	8.48	0.00	0.00
45	40.63	7.80	7.80	0.00	0.00
50	37.65	7.23	7.23	0.00	0.00
55	35.12	6.74	6.74	0.00	0.00
60	32.94	6.32	6.32	0.00	0.00
65	31.04	5.96	5.96	0.00	0.00

Storage: Surface Storage In and Above CB  
 ICD: HF ICD (Circular Orifice)  
 Orifice Equation:  $CdA(2gh)^{0.5}$  Where C = 0.572  
 Orifice Diameter: 79.00 mm  
 Invert Elevation: 136.68 m  
 T/G Elevation: 137.88 m  
 Max Ponding Depth: 0.26 m  
 Downstream W/L: 136.68 m

Stage	Head (m)	Discharge (L/s)	Vreq (cu. m)	Vavail (cu. m)	Volume Check
5-year Water Level	138.14	1.46	15.01	5.76	OK

**100 YEAR Modified Rational Method for Entire Site**

Subdrainage Area: C100A  
 Area (ha): 0.09  
 C: 0.95

Controlled - Tributary

tc (min)	I (100 yr) (mm/hr)	Qactual (L/s)	Qrelease (L/s)	Qstored (L/s)	Vstored (m³)	Qspill (L/s)
10	178.56	42.85	15.01	9.60	5.76	18.25
20	119.95	28.78	15.01	4.80	5.76	8.98
30	91.87	22.05	15.01	3.20	5.76	3.84
40	75.15	18.03	15.01	2.40	5.76	0.63
50	63.95	15.35	15.01	0.34	1.02	0.00
60	55.89	13.41	13.41	0.00	0.00	0.00
70	49.79	11.95	11.95	0.00	0.00	0.00
80	44.99	10.80	10.80	0.00	0.00	0.00
90	41.11	9.87	9.87	0.00	0.00	0.00
100	37.90	9.10	9.10	0.00	0.00	0.00
110	35.20	8.45	8.45	0.00	0.00	0.00
120	32.89	7.89	7.89	0.00	0.00	0.00

Storage: Surface Storage In and Above CB  
 ICD: HF ICD (Circular Orifice)  
 Orifice Equation:  $CdA(2gh)^{0.5}$  Where C = 0.572  
 Orifice Diameter: 79.00 mm  
 Invert Elevation: 136.68 m  
 T/G Elevation: 137.88 m  
 Max Ponding Depth: 0.26 m  
 Downstream W/L: 136.68 m

Stage	Head (m)	Discharge (L/s)	Vreq (cu. m)	Vavail (cu. m)	Volume Check
100-year Water Level	138.14	1.46	15.01	5.76	OK

Subdrainage Area: UNC-2  
 Area (ha): 0.01  
 C: 0.71

Uncontrolled - Non-Tributary

tc (min)	I (5 yr) (mm/hr)	Qactual (L/s)	Qrelease (L/s)
10	104.19	3.07	3.07
15	83.56	2.46	2.46
20	70.25	2.07	2.07
25	60.90	1.79	1.79
30	53.93	1.59	1.59
35	48.52	1.43	1.43
40	44.18	1.30	1.30
45	40.63	1.20	1.20
50	37.65	1.11	1.11
55	35.12	1.03	1.03
60	32.94	0.97	0.97
65	31.04	0.91	0.91

Subdrainage Area: UNC-2  
 Area (ha): 0.01  
 C: 0.89

Uncontrolled - Non-Tributary

tc (min)	I (100 yr) (mm/hr)	Qactual (L/s)	Qrelease (L/s)
10	178.56	6.57	6.57
20	119.95	4.41	4.41
30	91.87	3.38	3.38
40	75.15	2.76	2.76
50	63.95	2.35	2.35
60	55.89	2.06	2.06
70	49.79	1.83	1.83
80	44.99	1.65	1.65
90	41.11	1.51	1.51
100	37.90	1.39	1.39
110	35.20	1.29	1.29
120	32.89	1.21	1.21

Subdrainage Area: UNC-1  
 Area (ha): 0.05  
 C: 0.69

Uncontrolled - Non-Tributary

tc (min)	I (5 yr) (mm/hr)	Qactual (L/s)	Qrelease (L/s)
10	104.19	10.98	10.98
15	83.56	8.81	8.81
20	70.25	7.41	7.41
25	60.90	6.42	6.42
30	53.93	5.69	5.69
35	48.52	5.11	5.11
40	44.18	4.66	4.66
45	40.63	4.28	4.28
50	37.65	3.97	3.97
55	35.12	3.70	3.70
60	32.94	3.47	3.47
65	31.04	3.27	3.27

Subdrainage Area: UNC-1  
 Area (ha): 0.05  
 C: 0.86

Uncontrolled - Non-Tributary

tc (min)	I (100 yr) (mm/hr)	Qactual (L/s)	Qrelease (L/s)
10	178.56	23.53	23.53
20	119.95	15.81	15.81
30	91.87	12.11	12.11
40	75.15	9.90	9.90
50	63.95	8.43	8.43
60	55.89	7.37	7.37
70	49.79	6.56	6.56
80	44.99	5.93	5.93
90	41.11	5.42	5.42
100	37.90	4.99	4.99
110	35.20	4.64	4.64
120	32.89	4.33	4.33



# Stormwater Management Calculations

Project #160402061, 124 Moore Street Addition  
Modified Rational Method Calculations for Storage

SUMMARY TO OUTLET			
		Vrequired	Vavailable*
Tributary Area	0.091 ha		
Total 5yr Flow to Sewer	15.0 L/s	3.0	5.8 m <sup>3</sup>
Non-Tributary Area	0.070 ha		
Total 5yr Flow Uncontrolled	14.0 L/s		
Total Area	0.161 ha		
Total 5yr Flow	29.1 L/s		
Target	30.3 L/s		

SUMMARY TO OUTLET			
		Vrequired	Vavailable*
Tributary Area	0.091 ha		
Total 100yr Flow to Sewer	15.0 L/s		5.8 m <sup>3</sup>
Non-Tributary Area	0.070 ha		
Total 100yr Flow Uncontrolled	48.3 L/s		
Total Area	0.161 ha		
Total 100yr Flow	63.3 L/s		
Target	64.8 L/s		



**JOB NAME**  
 DATE: 2024-10-13  
 REVISION: 1  
 DESIGNED BY: ALD  
 CHECKED BY: SB

**STORM SEWER DESIGN SHEET**  
 (City of Ottawa)  
 FILE NUMBER: 160402061

**DESIGN PARAMETERS**  
 (As per City of Ottawa Guidelines, 2012)  
 $I = a / (b + I^2)$   
 a = 732.951 098.071 1174.184 1735.688  
 b = 6.199 6.653 6.014 6.014  
 c = 0.910 0.914 0.916 0.920  
 MANNING'S n = 0.013  
 MINIMUM COVER: 2.00 m  
 TIME OF ENTRY: 10 min  
 BEDDING CLASS = B

AREA ID NUMBER	FROM STA.	TO STA.	AREA (5-YEAR)	AREA (5-YEAR)	AREA (10-YEAR)	AREA (10-YEAR)	AREA (ROOF)	C (1-YEAR)	C (5-YEAR)	C (10-YEAR)	C (10-YEAR)	A+C (5-YEAR)	ACCUM (5-YEAR)	A+C (10-YEAR)	ACCUM (10-YEAR)	A+C (10-YEAR)	ACCUM (10-YEAR)	T+C	V <sub>50%</sub>	V <sub>75%</sub>	V <sub>85%</sub>	V <sub>90%</sub>	Q <sub>50%</sub>	Q <sub>75%</sub>	Q <sub>85%</sub>	Q <sub>90%</sub>	LENGTH (m)	PIPE DIAMETER (mm)	PIPE HEIGHT (mm)	PIPE SHAPE	MATERIAL	PIPE SELECTION									
																																CLASS	SLOPE (%)	Q <sub>10%</sub> (L/S)	% FULL (S/A)	V <sub>10%</sub> (m/s)	% FULL (F/L)	V <sub>10%</sub> (m/s)	V <sub>10%</sub> (m/s)	TIME OF FLOW (min)	
C100A	CB C100A-1 SEWER		0.00	0.10	0.00	0.00	0.00	0.00	0.76	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	10.00	76.81	104.19	122.14	178.56	15.0	15.0	15.0	6.3	200	200	CIRCULAR	PVC	-	2.65	54.2	27.6%	1.71	1.21	0.09			



Stantec Consulting Ltd.  
300 + 101 Queen Avenue  
Ottawa, ON  
M4 4J3 7Z2 4A20  
www.stantec.com

Copyright Reserved  
The Client is the owner and is responsible for the information provided. Stantec is not responsible for the information provided. The Client is responsible for the accuracy of the information provided. Stantec is not responsible for the accuracy of the information provided.

- Legend**
- AREA ID
  - EXISTING PLANT COMPONENT
  - EXISTING STORM DRAINAGE AREA NO.
  - EXISTING STORM DRAINAGE BOUNDARY
  - EXISTING STORM DRAINAGE DRAINAGE POINT
  - EXISTING STORM DRAINAGE
  - EXISTING CURB/STREET
  - EXISTING ELEVATION
  - EXISTING PROPOSED ELEVATION
  - EXISTING DEPRESSION

**Notes**

Revision	Description	Date	By	Check
1	ISSUED FOR PERMIT	2024	J.S.	J.S.
2	REVISED FOR DEVELOPER APPROVAL	2024	J.S.	J.S.
3	REVISED FOR DEVELOPER APPROVAL	2024	J.S.	J.S.

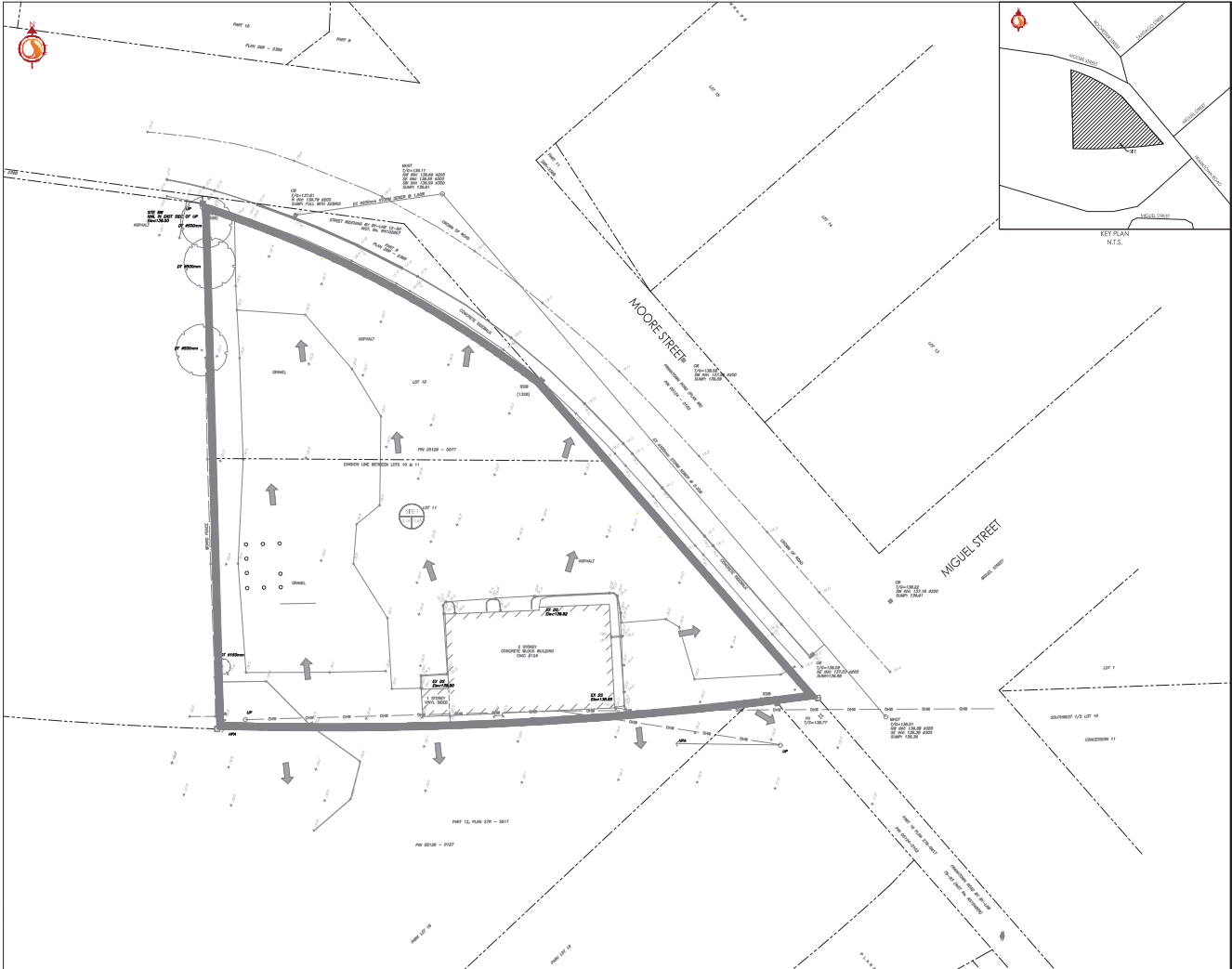
Form-Sed

**Client/Project**  
SPURLINE DENTAL  
124 MOORE STREET  
Collingwood, ON

**Title**  
EXISTING CONDITIONS  
SITE DRAINAGE PLAN

Project No. 16240268  
Drawing No. Sheet

Scale 1:100  
Revision 3



2024/07/23 10:00 AM



Copyright Reserved  
The Client and the Designer are responsible for the information provided in this drawing. The Designer is not responsible for the information provided in this drawing if it is not used in accordance with the terms of the contract. The Client and the Designer are responsible for the information provided in this drawing if it is not used in accordance with the terms of the contract.

**Legend**

- AREA ID
- RIGHT-OF-WAY BOUNDARY
- STORM DRAINAGE AREA BOUNDARY
- STORM DRAINAGE LINE
- PROPOSED STORM DRAINAGE LINE
- PROPOSED CATCHBASIN
- PROPOSED STORM INLET
- PROPOSED STORM MANHOLE
- STORM MANHOLE WITH COVER
- STORM MANHOLE WITH INLET
- PROPOSED STORM INLET
- PROPOSED STORM MANHOLE

**Notes**

1. THIS DRAWING IS FOR INFORMATION ONLY.
2. CONSULT THE ARCHITECTURAL AND STRUCTURAL DRAWINGS FOR DETAILED INFORMATION.
3. REFER TO ARCHITECTURAL AND STRUCTURAL DRAWINGS FOR DETAILED INFORMATION.
4. REFER TO ARCHITECTURAL AND STRUCTURAL DRAWINGS FOR DETAILED INFORMATION.

Revision	Date	By	Appr.	Drawn	Checked
1	2023-10-20	JL	MS	JL	MS

**Form/Issued**

Client/Project  
SPURLINE DENTAL

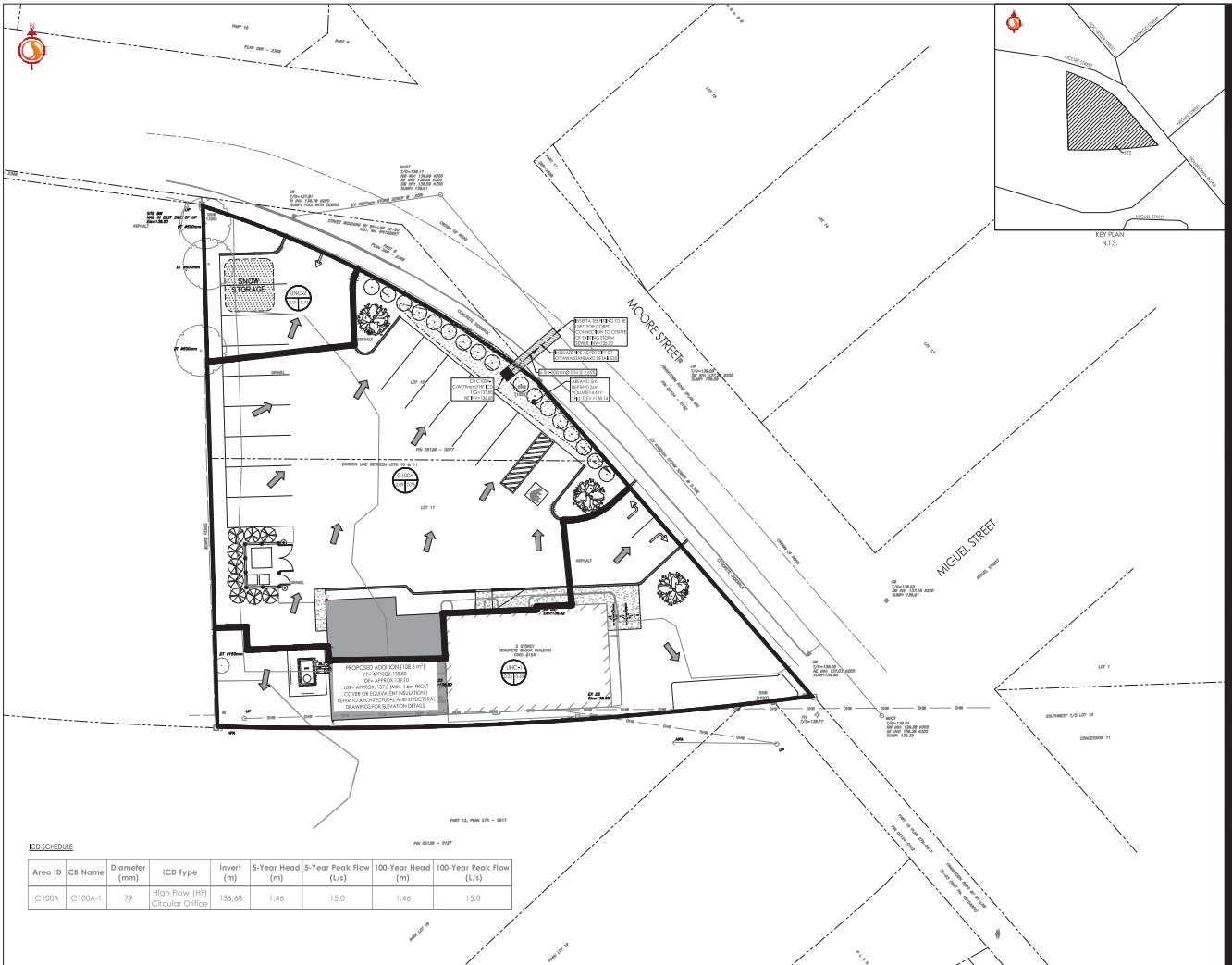
124 MOORE STREET

Coleton Place, ON

Title  
SITE DRAINAGE PLAN

Project No. 16240261  
Drawing No. Sheet  
Scale: 1" = 10'-0"

Revision  
SD-1 3 of 3 3



**ICD SCHEDULE**

Area ID	CB Name	Diameter (mm)	ICD Type	Invert (m)	5-Year Head (m)	5-Year Peak Flow (L/s)	100-Year Head (m)	100-Year Peak Flow (L/s)
C100A	C100A-1	79	High Flow (HF) Circular Inlet	136.68	1.46	15.0	1.46	15.0